



University
of Glasgow

Micro-scale modelling of internally unstable soils

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Outline

- Internal instability
- Micro-scale modelling
- Hydromechanical criteria for erosion:
 - Stress reduction in finer particles
 - Coupled DEM-CFD Analysis

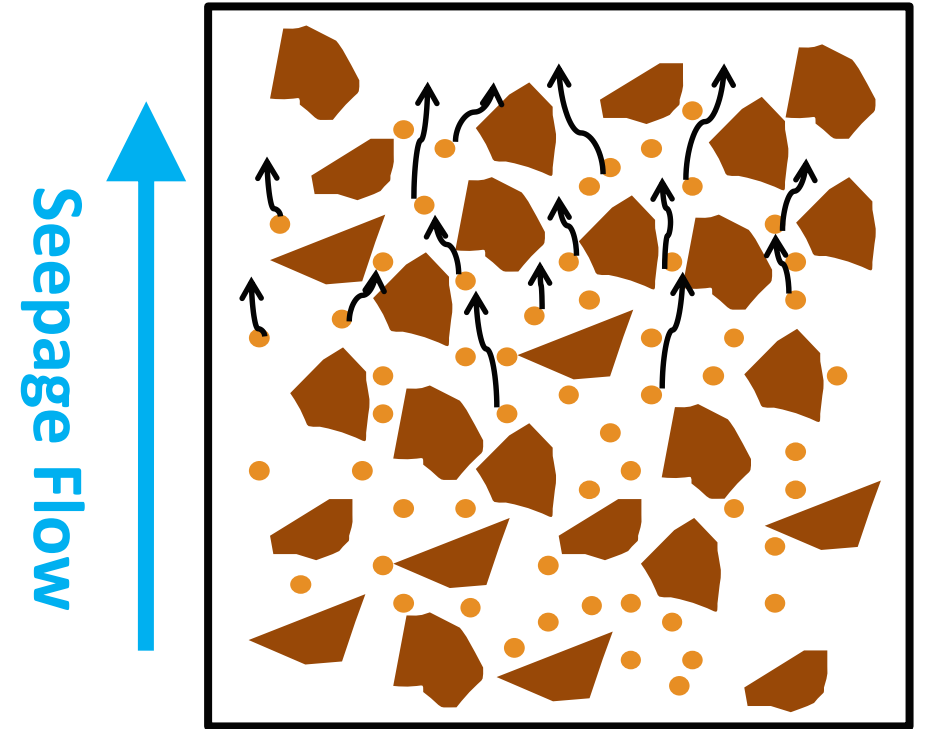
Internal Instability

Gap/broadly graded soils: Fine fraction preferentially eroded

Erosion of fines at **low hydraulic gradients** ($i < 0.3$)

Prerequisites:

- Cohesionless fines
- Load carrying matrix of coarse particles
- Fines under low stress
- Fines small enough to pass between coarse



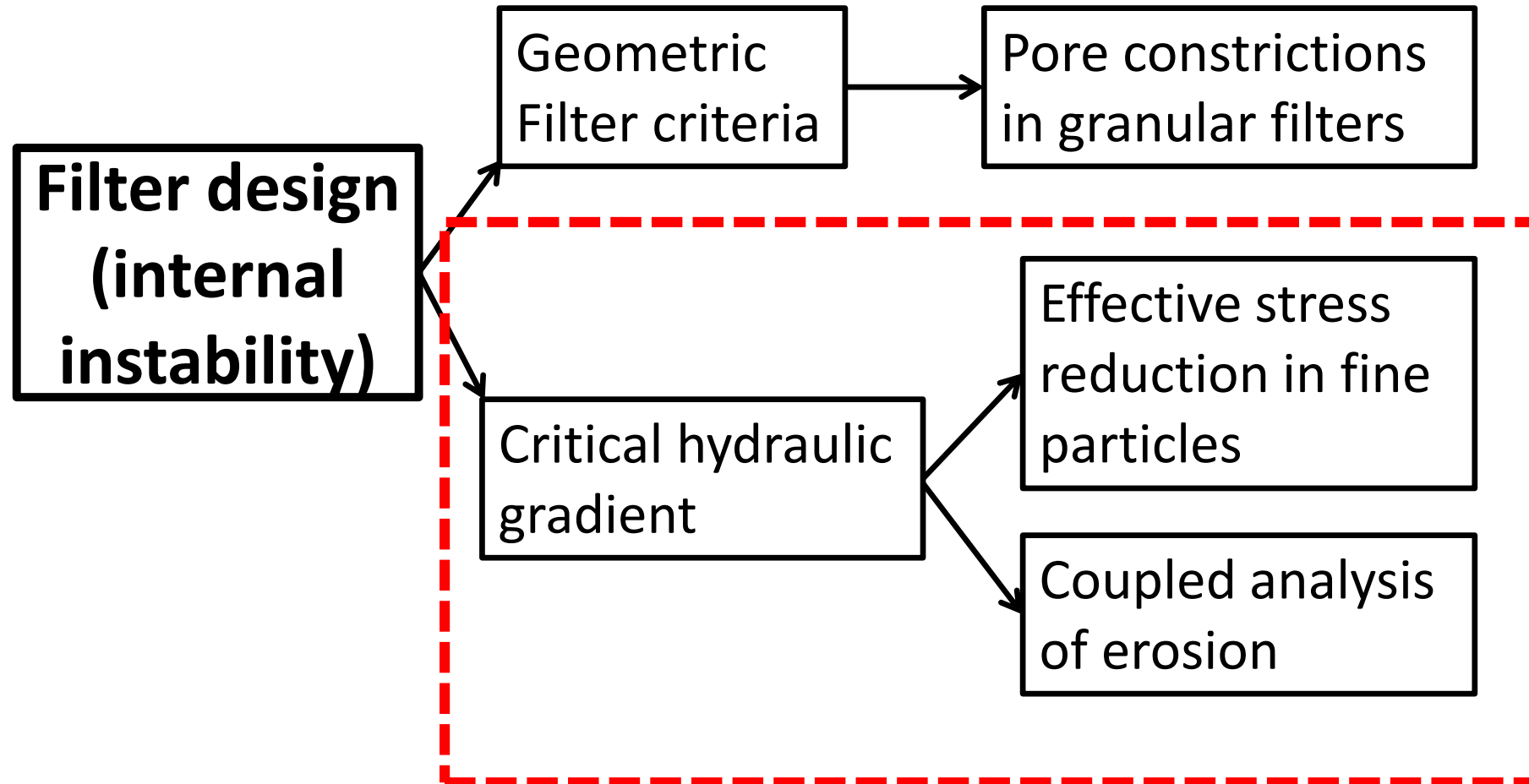
Eurocode 7: Hydraulic Failure

“[Geometric] **Filter criteria** shall be used to limit the danger of material transport by internal erosion.”

“If the filter criteria are not satisfied, the **hydraulic gradient** should be well below the critical gradient at which soil particles begin to move.”

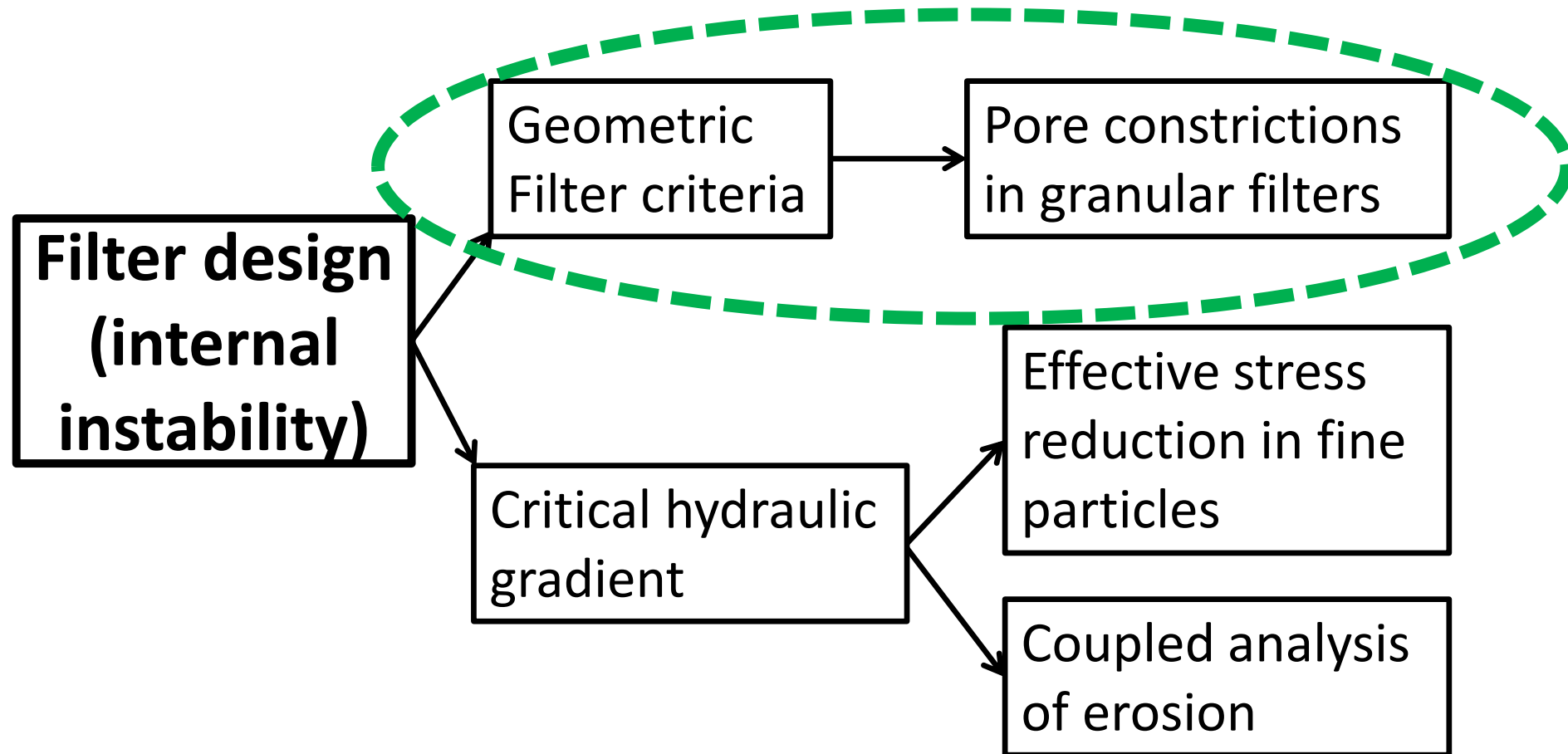
Aim of micro-scale analysis

Examine both elements of filter design:



Aim of micro-scale analysis

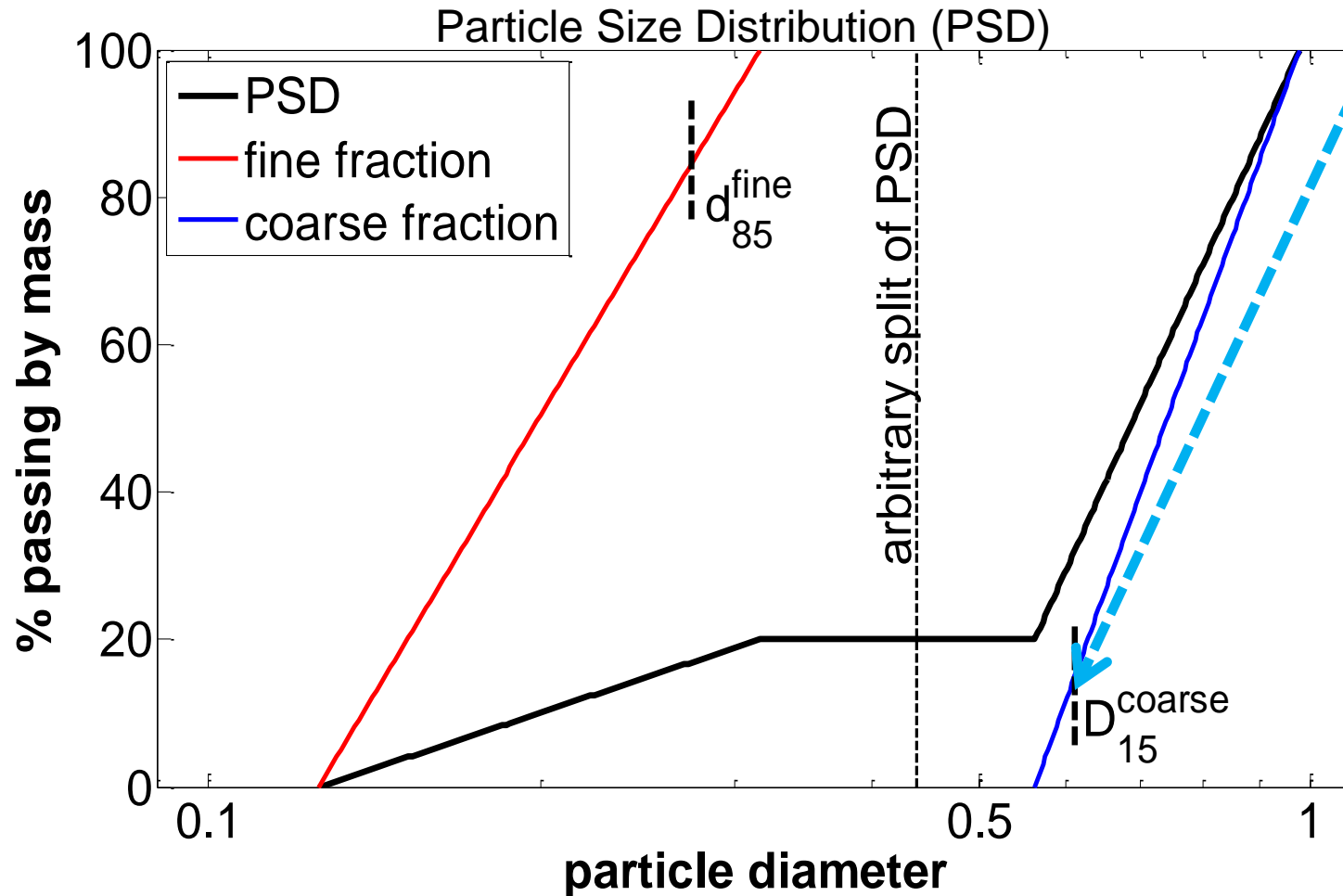
Examine both elements of filter design:



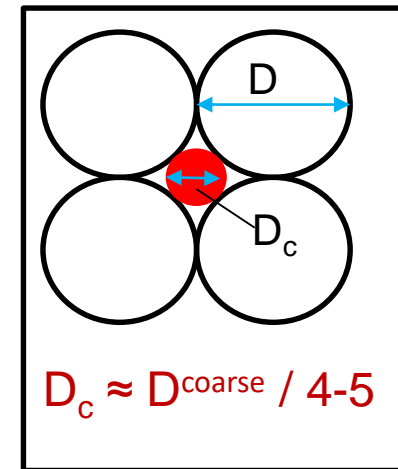
Geometric Filter Criteria: Kézdi (1979)

Split Gap-graded PSD into coarse and fine “PSDs”

Stable if: $D_{15}^{\text{coarse}} / d_{85}^{\text{fine}} < 4$



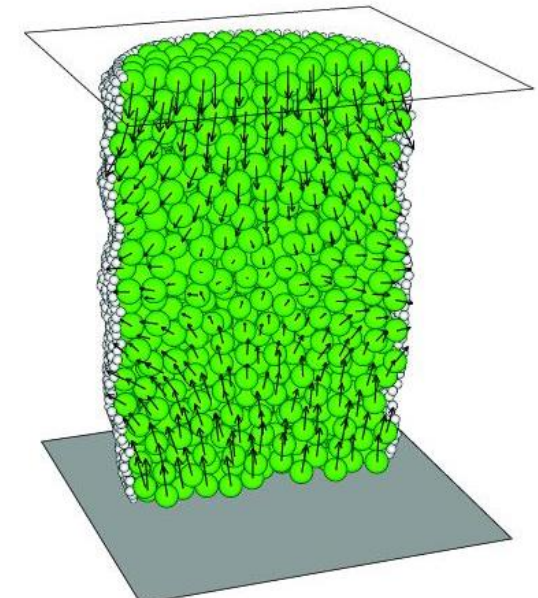
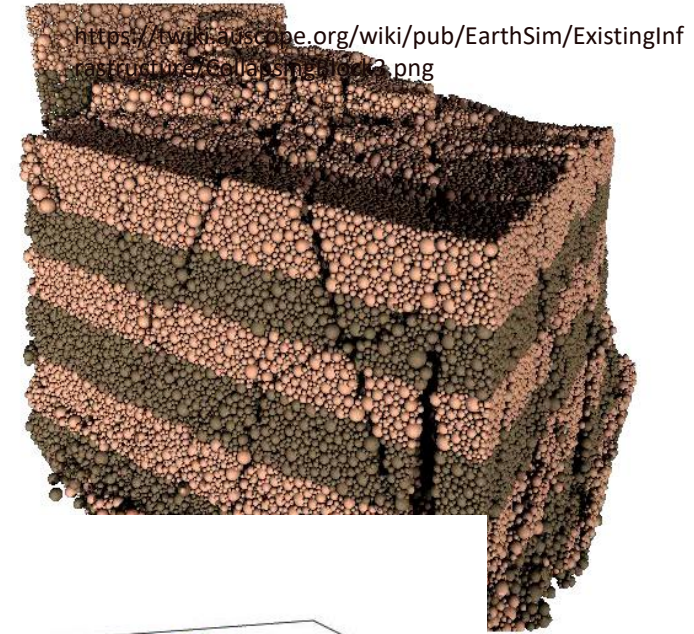
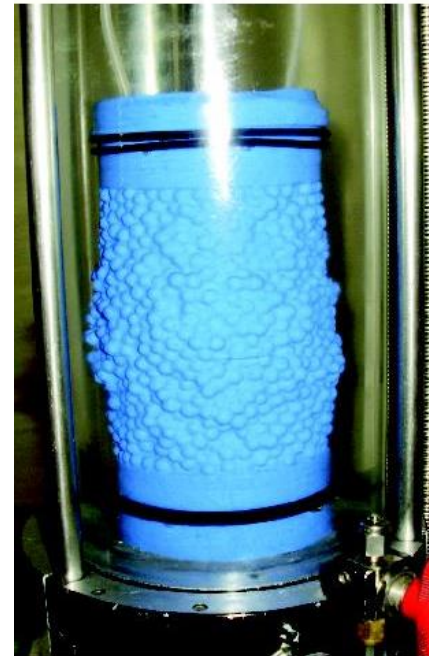
$(D_{15}^{\text{coarse}} / 4)$ proxy
for ‘pore size’



Micro-scale modelling:
Discrete Element Modelling

Micro-scale modelling

- DEM: Discrete Element Modelling
- Each element is a single soil particle
- Generally used for coarse-grained soils $>100\mu\text{m}$: Body forces dominate
- Used for virtual laboratory experiments

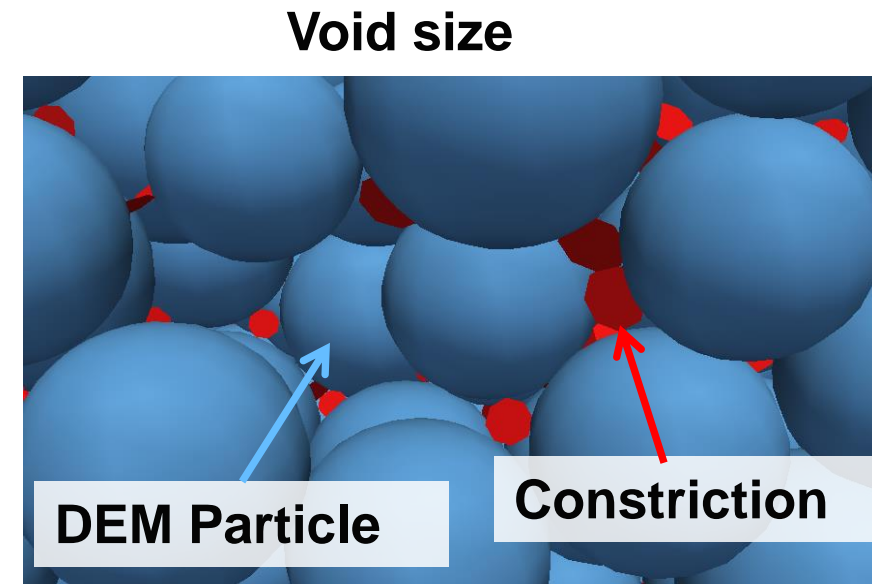
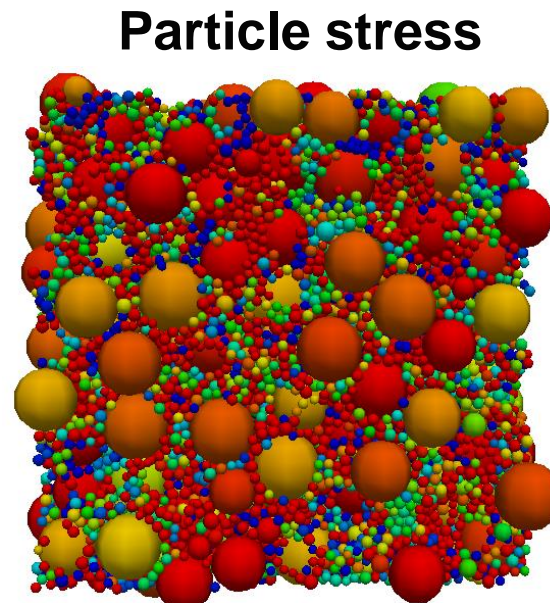
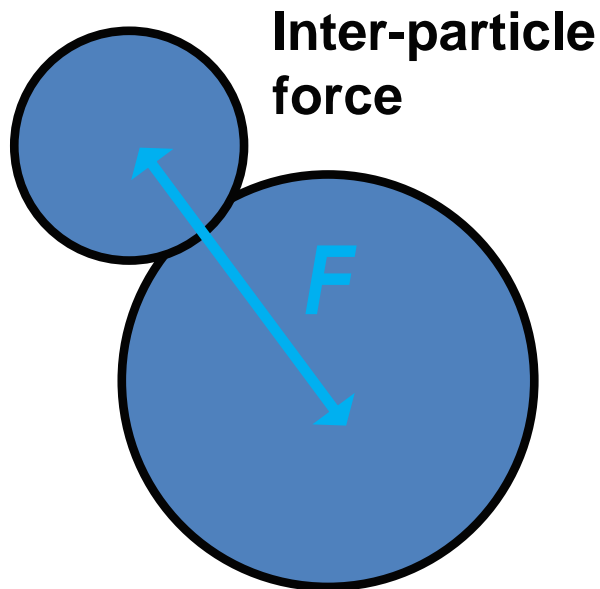


<http://web.utk.edu/~alshibli/research/MGM/DEM.php>

Discrete element method (DEM)

- Rigid particles which can overlap at contacts
- Overlap proportional to force acting at contacts
- Particles move at each timestep and make / break contact with neighbours

Measure variables unavailable in lab:



Hydromechanical potential for erosion : Stress
reduction in finer particles

Critical hydraulic gradient and stress

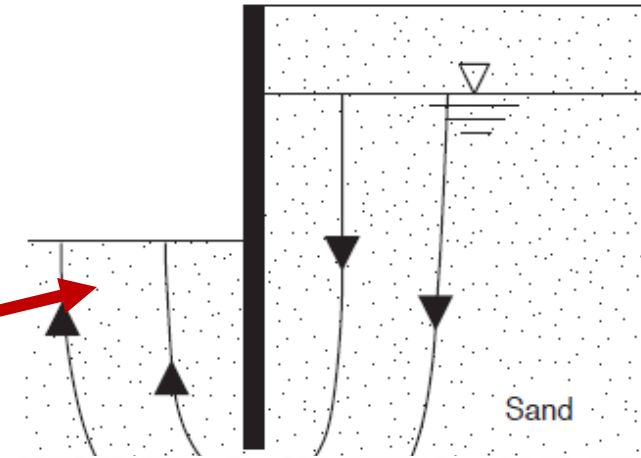
For internally stable soils:

$i_{\text{crit(heave)}}$: hydraulic gradient at which $\underline{\sigma_v}' = 0$

$$i_{\text{crit(heave)}} = \gamma' / \gamma_w \approx 1.0 \quad (\text{Terzaghi, 1925})$$

For internally unstable soils:

$$i_{\text{crit(unstable)}} < 1 \quad (\text{as low as } 0.2)$$



How to predict $i_{\text{crit(unstable)}}$??

Fig. 9. Material A: strong general piping of fines ($i = 0.22$, $v = 0.27$ cm/s)

Skempton and Brogan (1994)

Hydromechanical criterion

Skempton and Brogan (1994):

Coarse particles transfer overburden

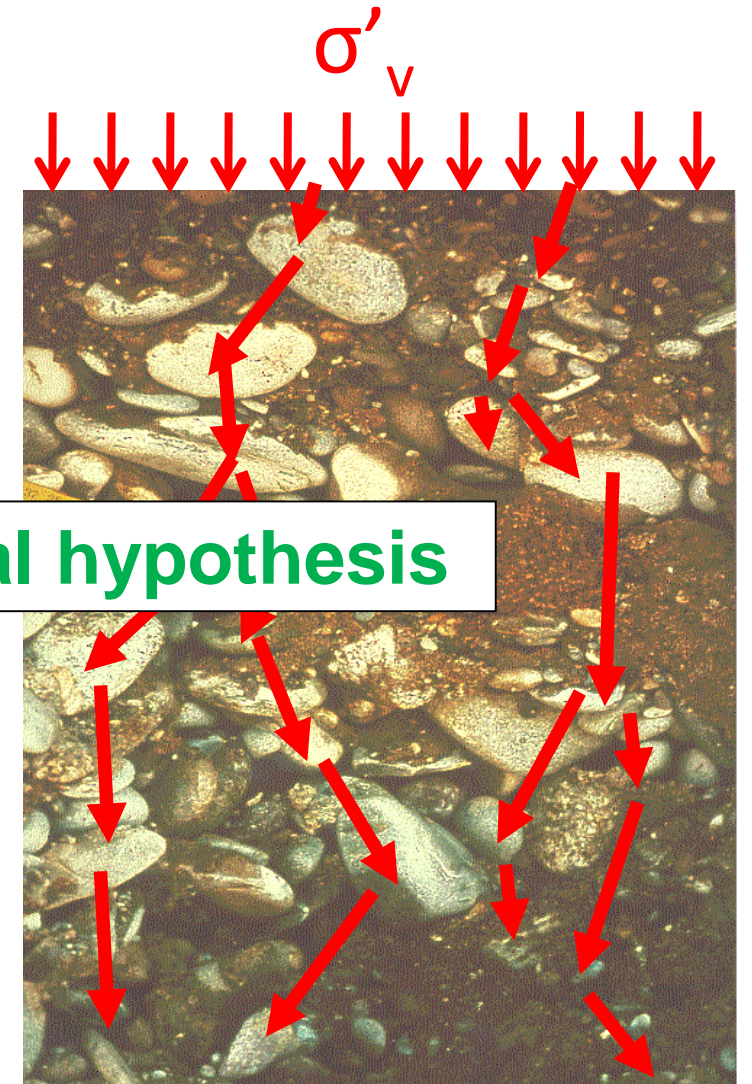
Fines carry reduced effective stress:

$$\sigma'_{\text{fine}} = \alpha \sigma'$$

Micromechanical hypothesis

Critical gradient to reach $\sigma' = 0$

$$i_{\text{crit}(\text{unstable})} = \alpha i_{\text{crit}(\text{heave})}$$



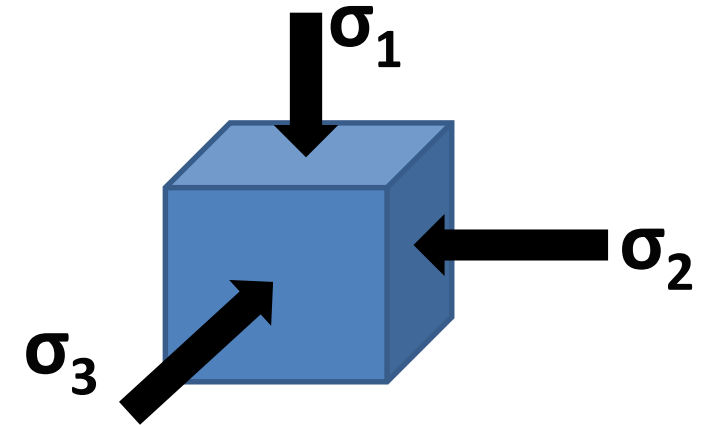
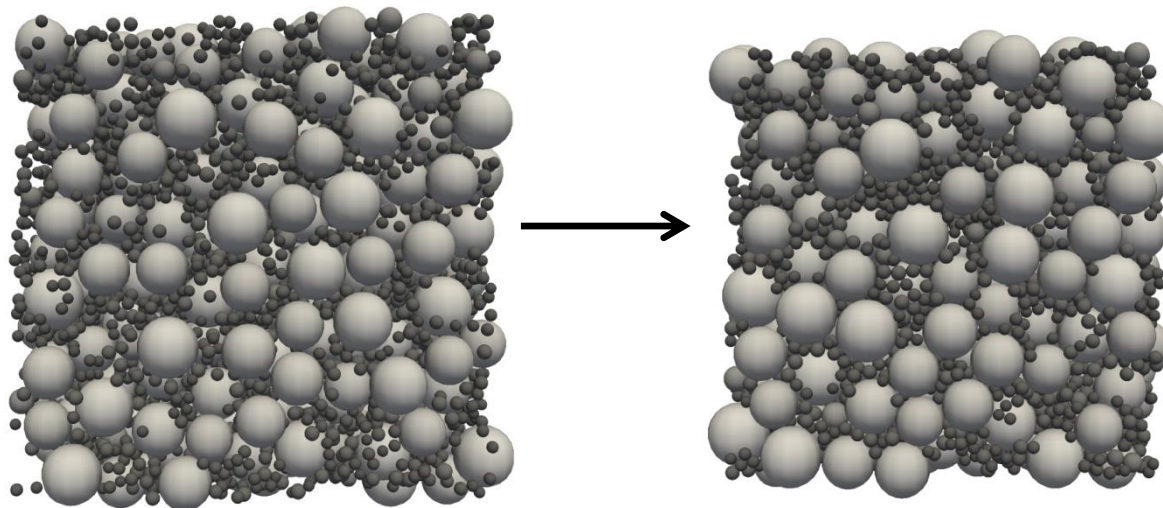
DEM Simulations

> 300,000 particles for large simulations

Servo-controlled **isotropic compression** to $p' = 50\text{kPa}$

Periodic cell – no boundary effects

Sample **density controlled** using interparticle friction:
 $\mu = 0.0$ (Dense), $\mu = 0.1$ (Medium dense), $\mu = 0.3$ (Loose)



Samples tested

Gap-graded samples

Study effect of:

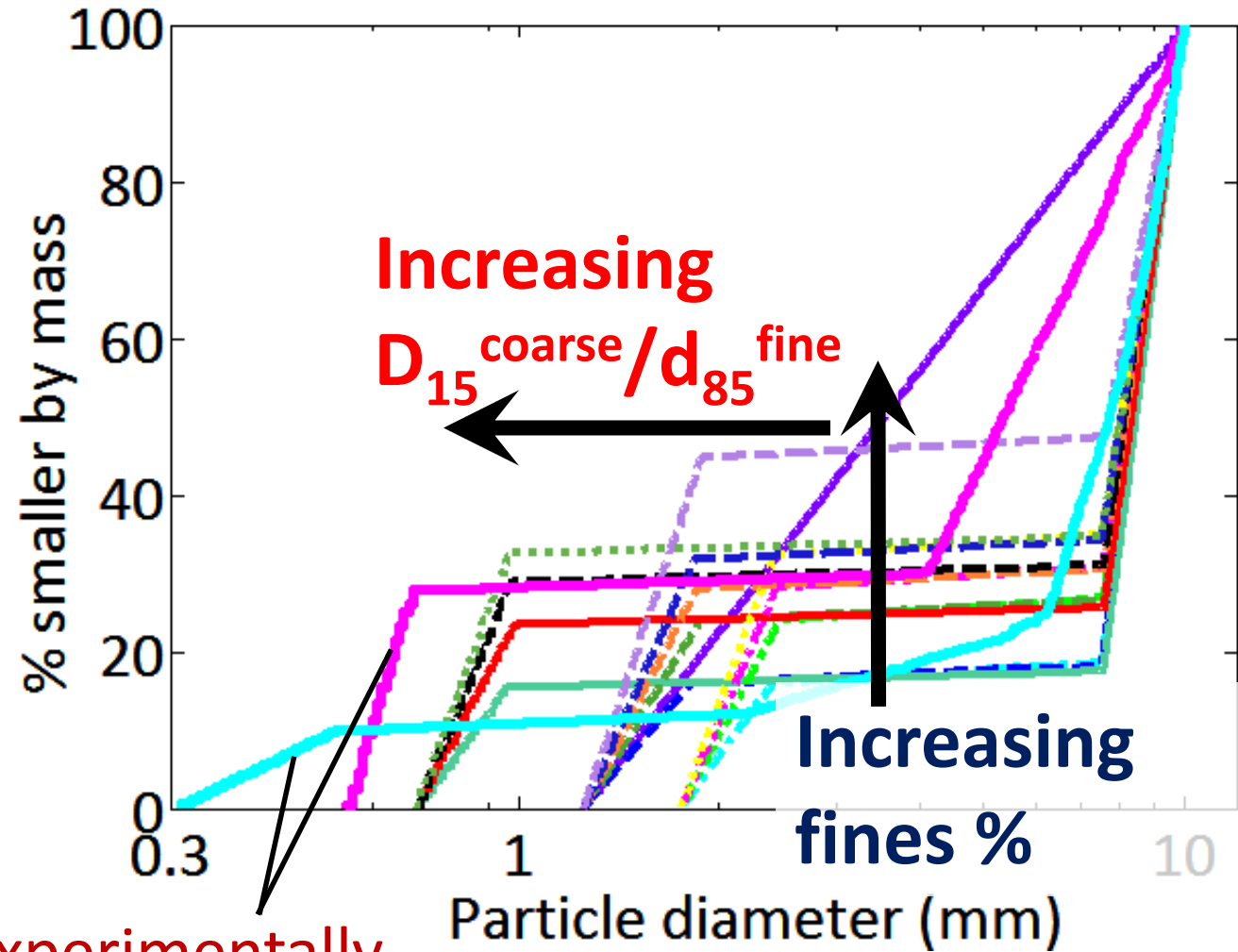
- PSD (gap-ratio + fine-content)
- Relative density

DEM: Measurement of α -factor:

$$\alpha = \frac{p'_{fine}}{p'}$$

Mean stress in finer particles

Mean stress in all particles



Two experimentally tested PSDs

Effect of % fines and density

Results

Low fines (<25%)

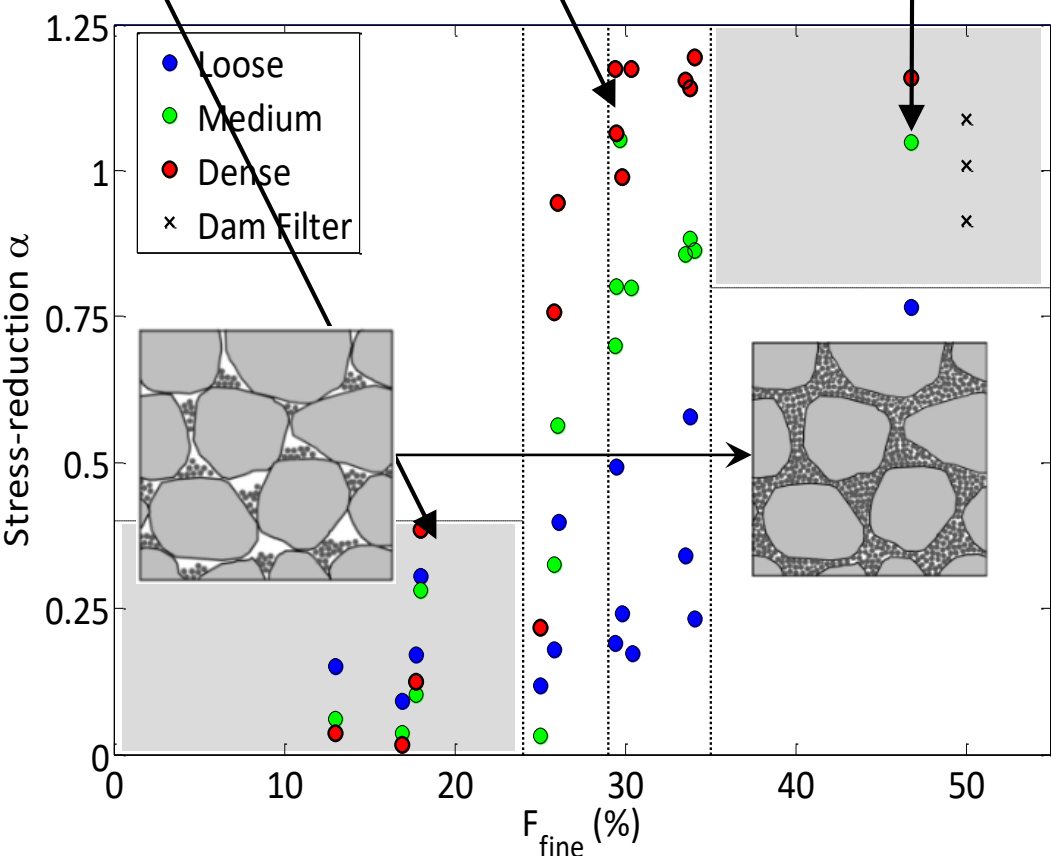
$\alpha < 0.4$

Fines don't fill voids

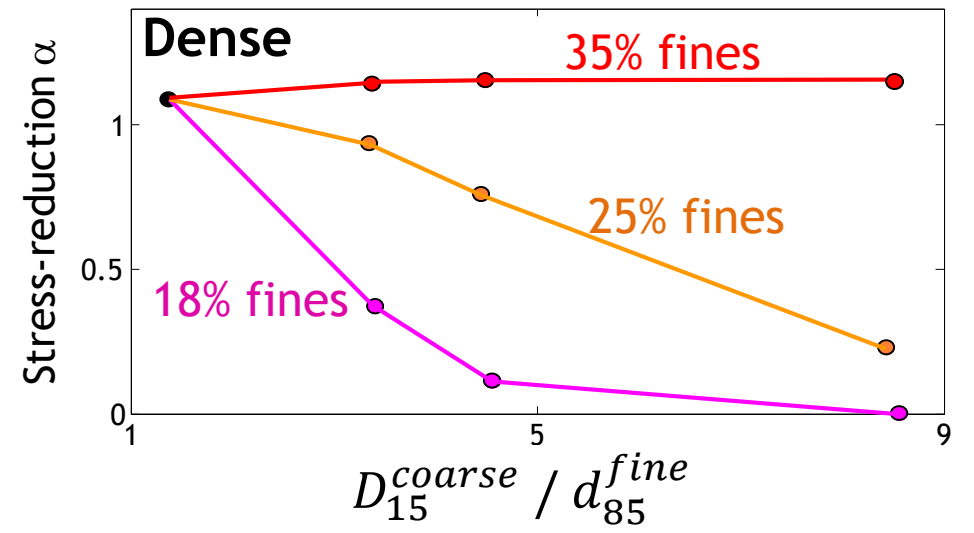
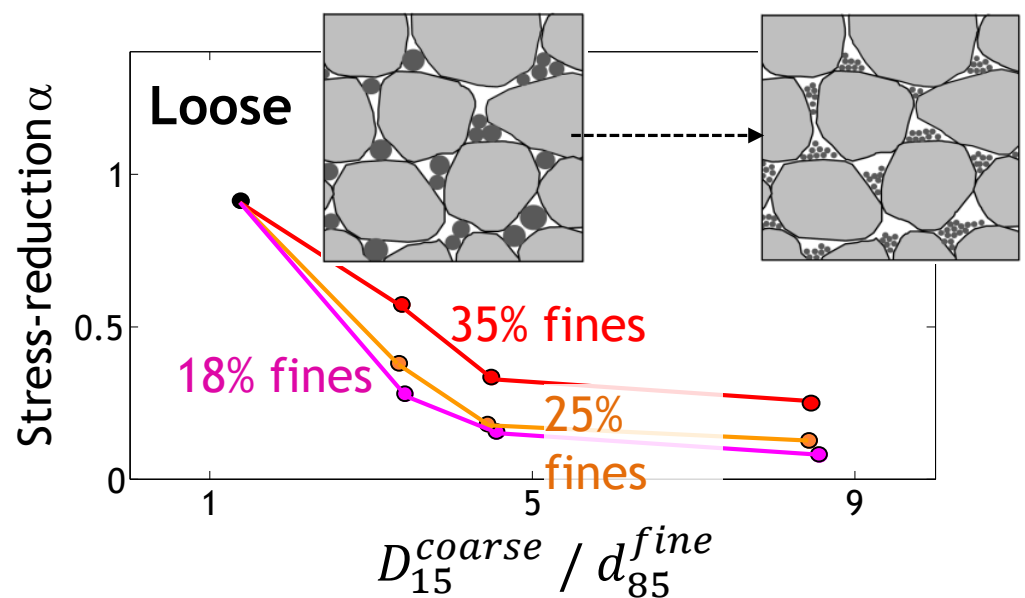
High fines (>35%); $\alpha \approx 1$

Fines separate coarse particles

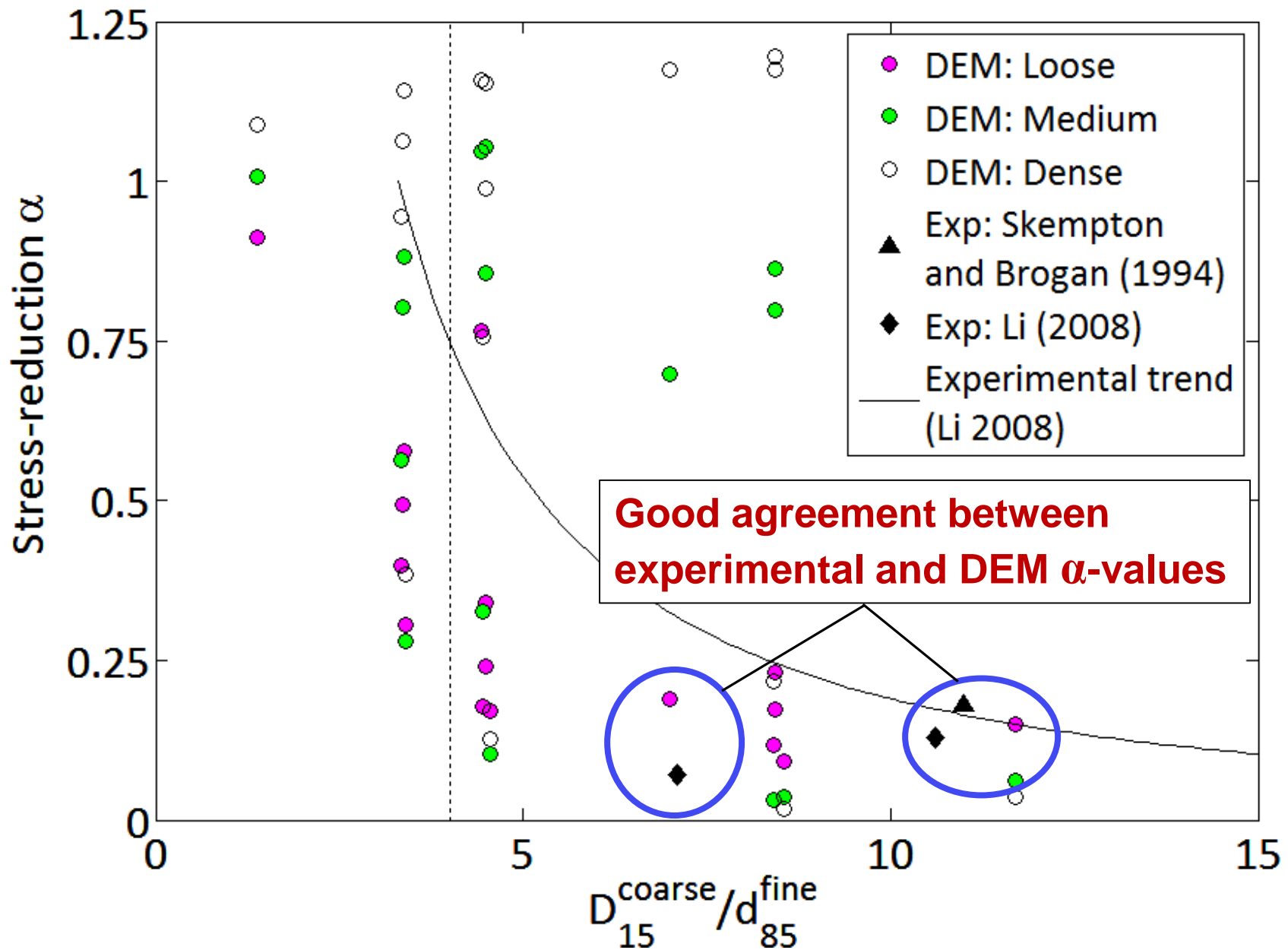
Intermediate:
Influence of relative density



Effect of gap-ratio

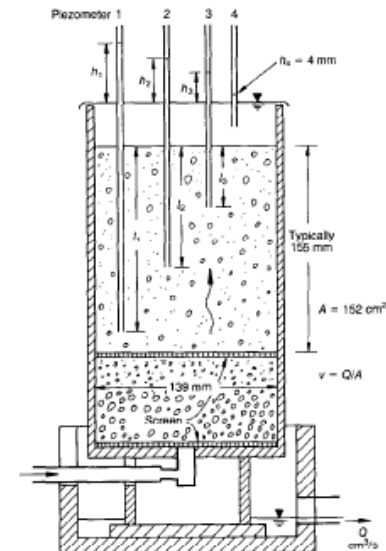


All Results

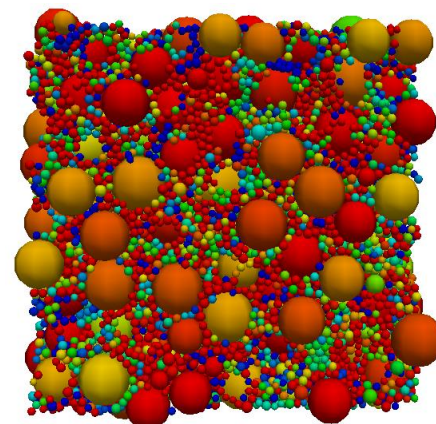


Experimental: Permeameter

$$\alpha = i_{crit(unsafe)} / i_{crit(heave)}$$



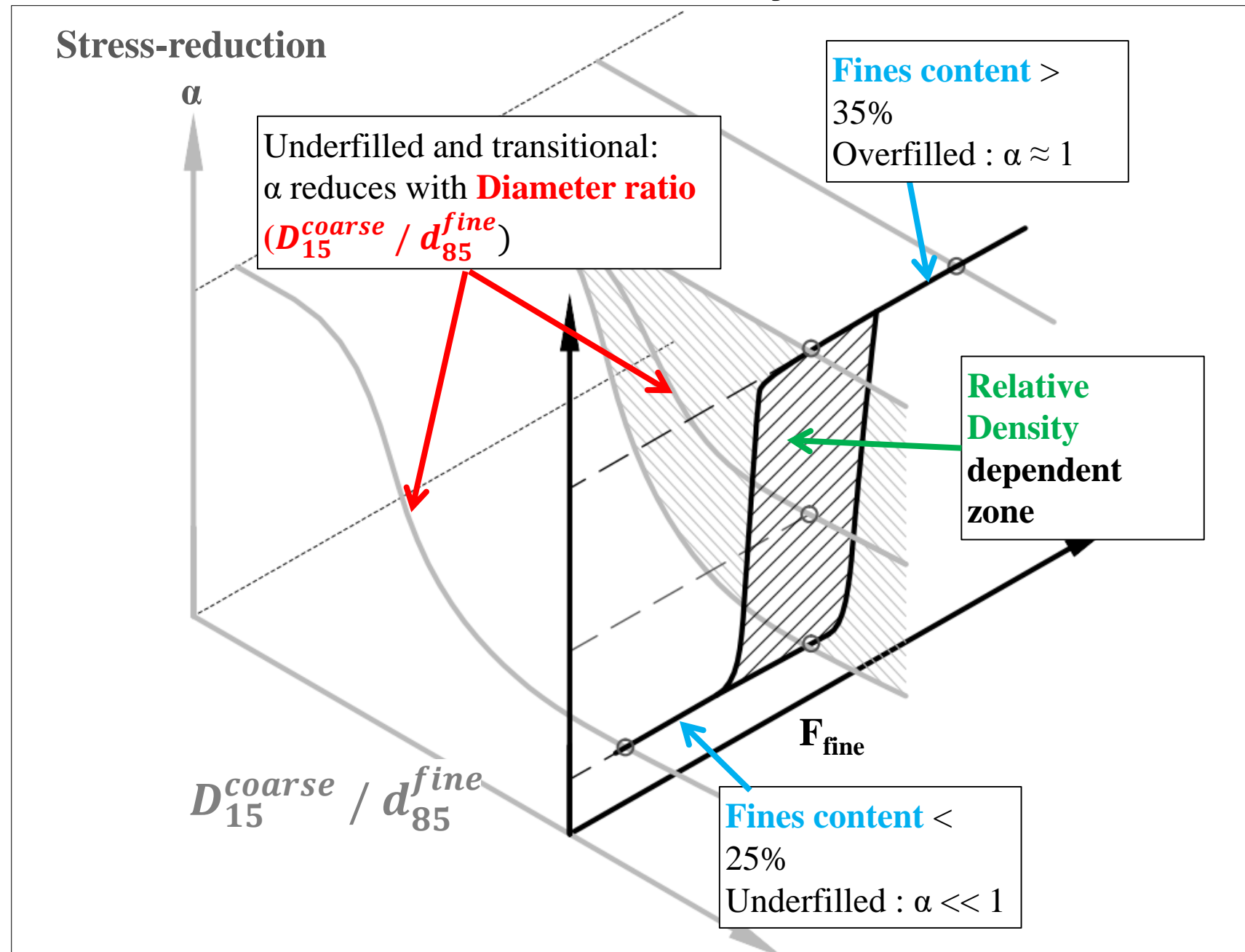
DEM: $\alpha = p'_{fine} / p'$



Stress reduction in fines : Summary

Three important factors:

- Fines content,
- Relative density
- Diameter ratio



Hydromechanical potential for erosion :

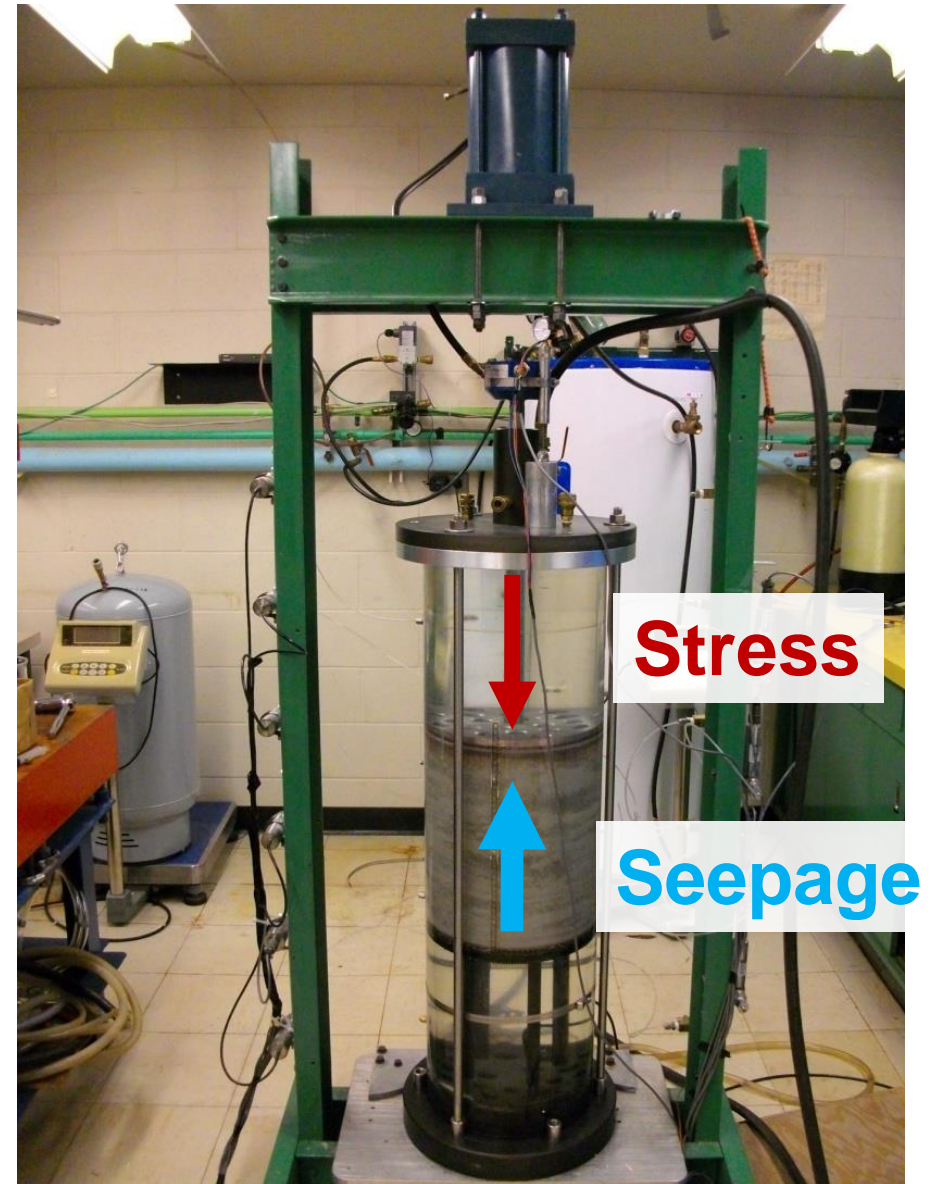
Coupled DEM-CFD analysis

Coupled DEM-CFD

Aim: model permeameter at micro-scale

Large permeameter for studying suffusion:

- Can vary top stress and hydraulic gradient
- Macro-scale measurements:
 - Hydraulic gradient (local)
 - Change in permeability
 - Visual observation of suffusion



University of British Columbia

Coupling DEM + Computational Fluid Dynamics

Soil-fluid interaction

- DEM: soil particles
- CFD: water seepage

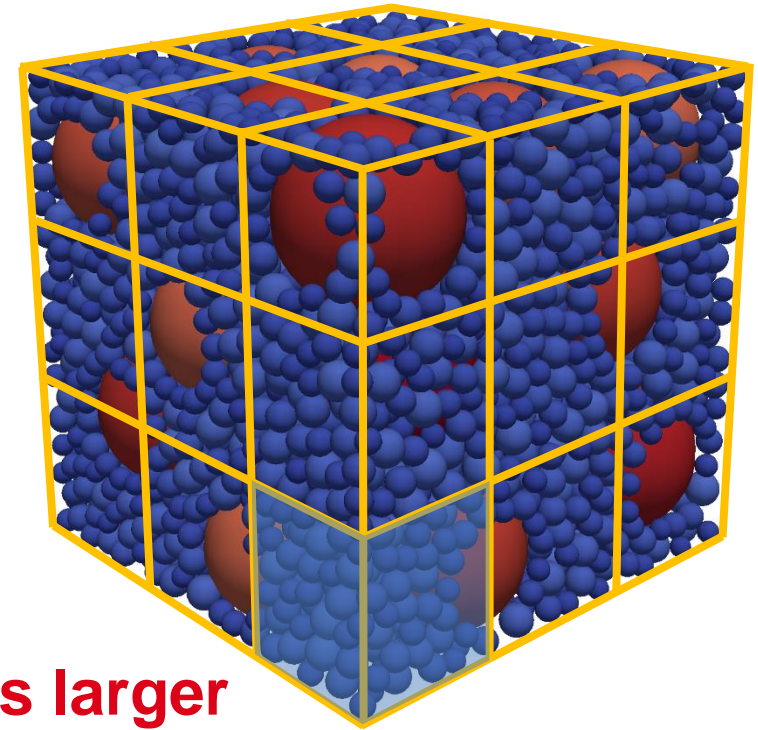
Data exchange



-Fluid velocity
-Fluid pressure
gradient

-Porosity
-Drag force

Coarse grid method
proposed by Tsuji



**Fluid cells larger
than particles**

(Tsuji et al., 1993, Xu and Yu, 1997)

Coarse grid method

Averaged Navier-Stokes equation

$$\underbrace{\frac{\partial n v_f}{\partial t} + v_f \nabla (n v_f)}_{\text{Inertia force}} = \underbrace{\frac{\mu_f}{\rho_f} \nabla (n \nabla v_f)}_{\text{Viscosity}} - \underbrace{\frac{n}{\rho_f} \nabla p}_{\text{Pressure gradient force}} + \underbrace{f_{drag}}_{\text{Fluid-particle interaction force}}$$

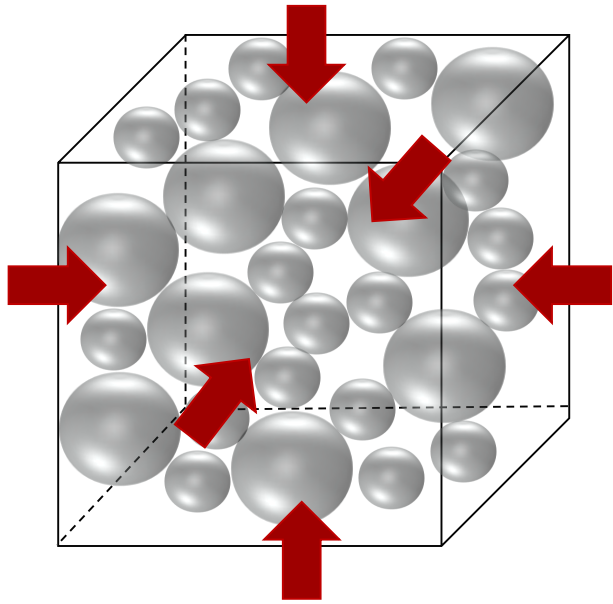
The equation is annotated with brackets and labels: a large bracket above the left side is labeled "Inertia force"; a bracket below the first term is labeled "Unsteady acceleration"; a bracket below the second term is labeled "Convective acceleration"; a bracket below the first term on the right is labeled "Viscosity"; a bracket below the second term on the right is labeled "Pressure gradient force"; and a bracket above the f_{drag} term is labeled "Fluid-particle interaction force".

n: porosity
 v_f : fluid velocity
 μ_f : fluid viscosity
 ρ_f : fluid density

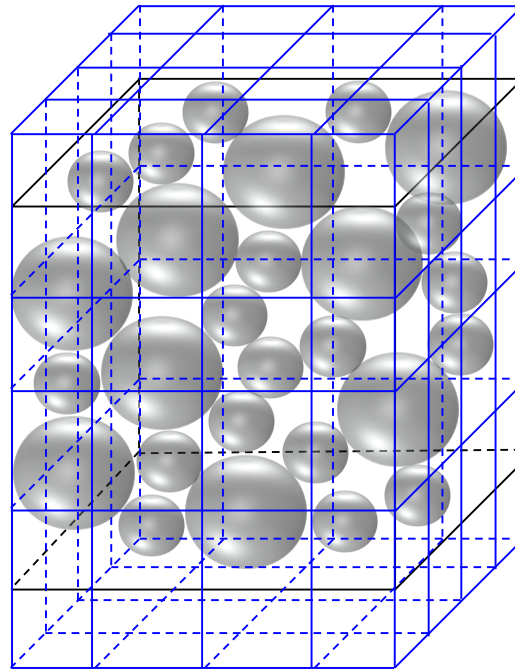
f_{drag} calculated using empirical equation based on fluid velocity, porosity and particle diameter (Di Felice, 1994)

Simulating permeameter tests

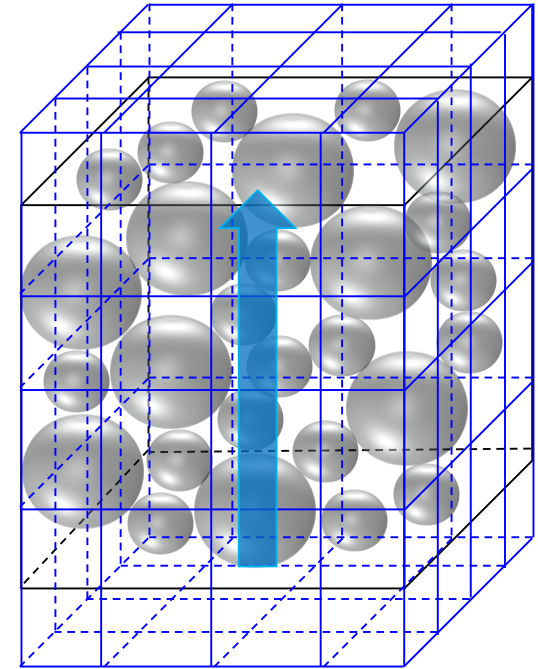
- Virtual stress-controlled permeameter



Compress to 50kPa,
Apply gravity



Create fluid mesh,
Fix boundaries,

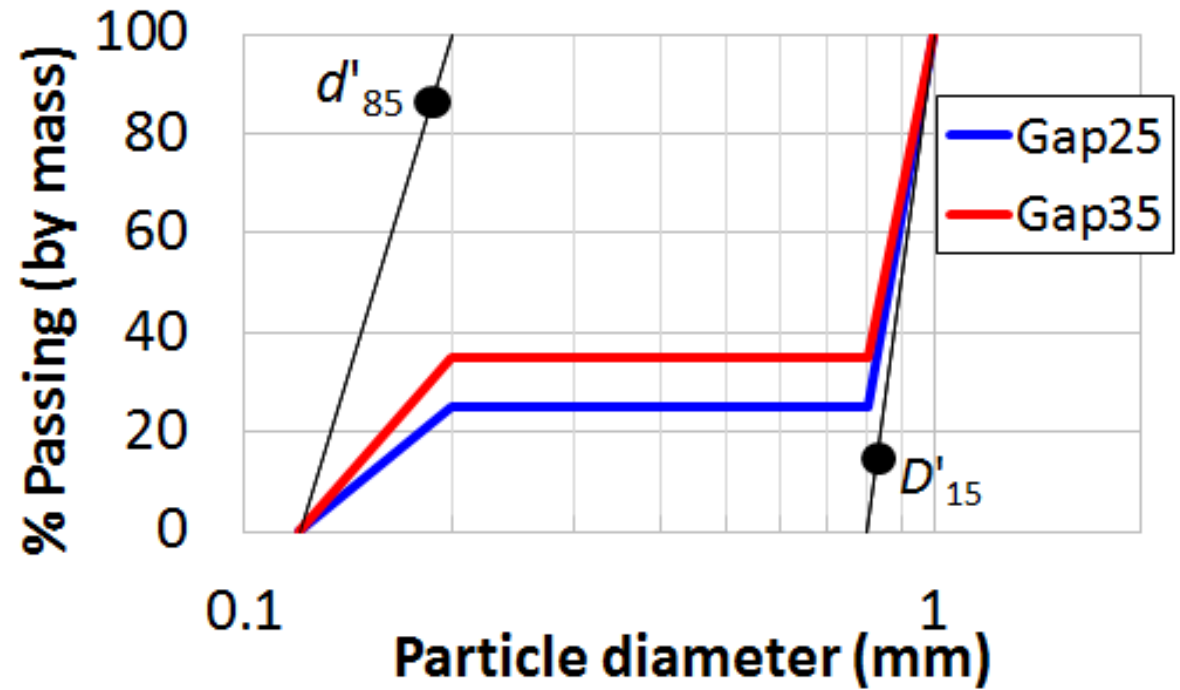


Apply pressure
gradient,
Monitor steady state
response

Virtual permeameter simulations

- 4 Samples tested: Up to 35000 particles
- $D_{15}^{\text{coarse}} / d_{85}^{\text{fine}} = 4.6$: Borderline unstable (Kezdi, 1979)
- 25% and 35% fines
 - Each Loose and Dense
- Hydraulic gradient, $i = 1, 2, 5, 10$
- $k \approx 1.0 \times 10^{-4} \text{ m/s}$ to $2.3 \times 10^{-4} \text{ m/s}$

Similar to experimental data from UBC

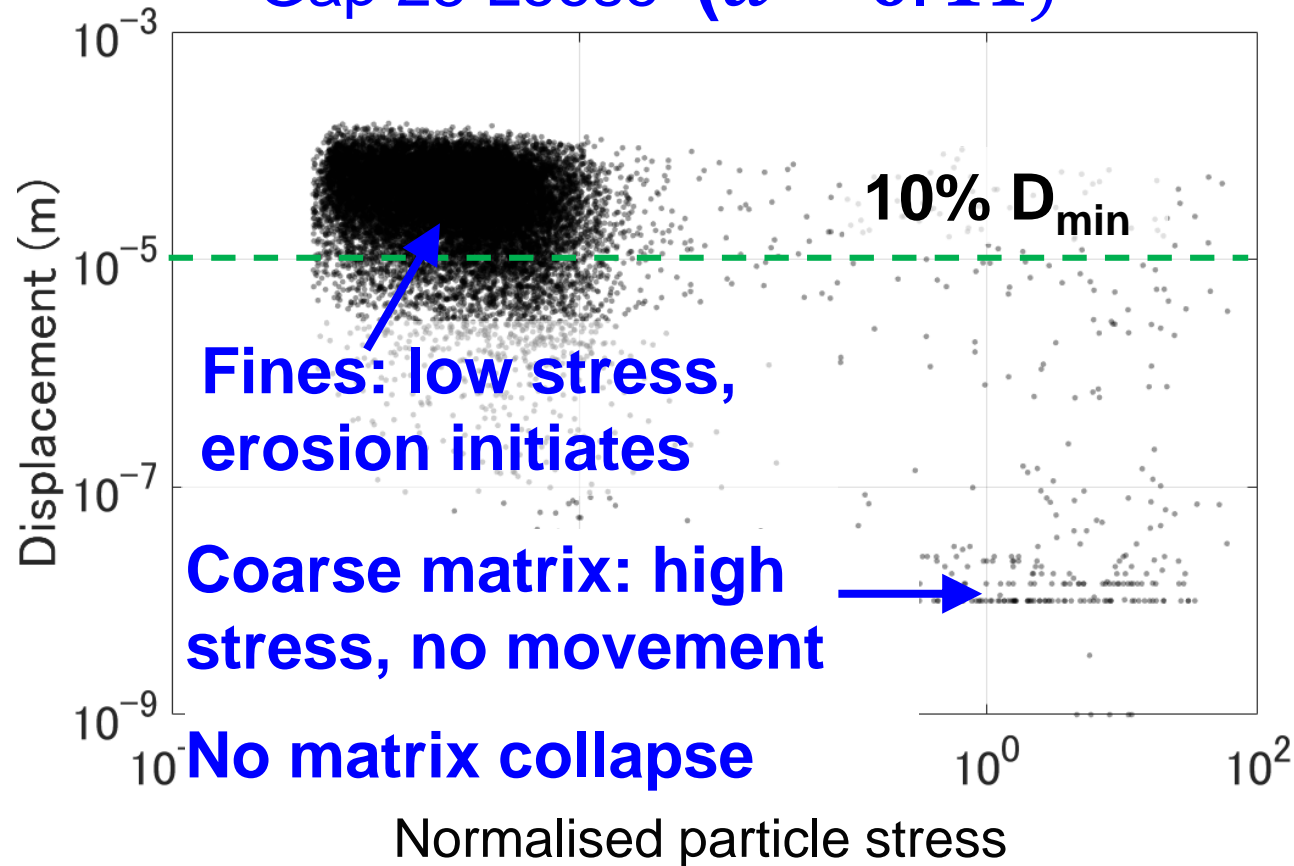


Coupled analysis: typical micro-scale result

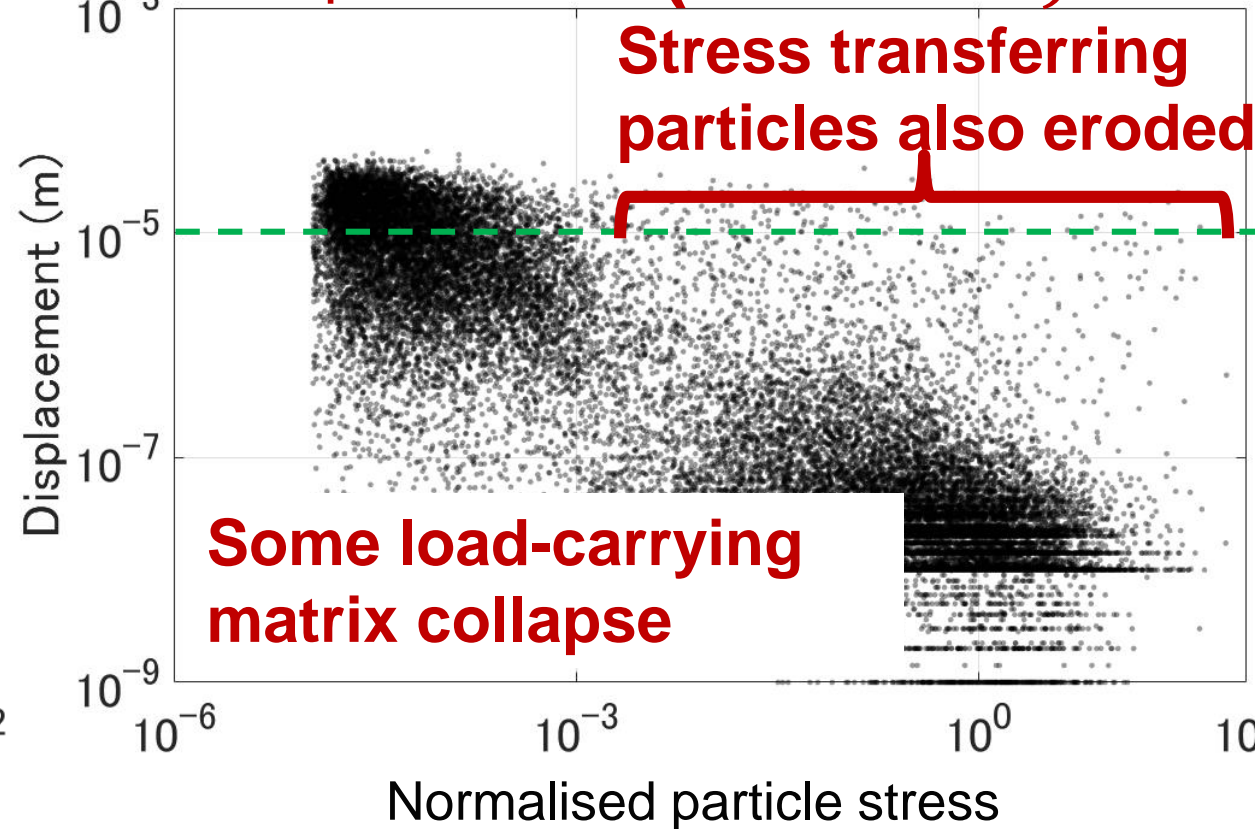
Hydraulic gradient: $i = 1$

$$\text{Normalised particle stress} = \frac{\sigma^{part}}{\sigma'}$$

Gap 25 Loose ($\alpha = 0.11$)



Gap 35 Loose ($\alpha = 0.34$)

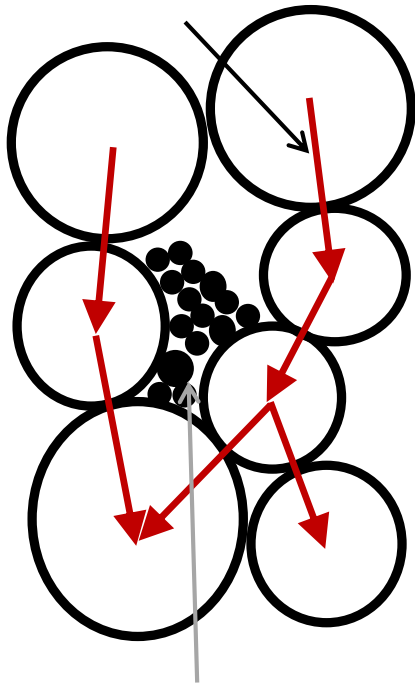


Coupled analysis: typical micro-scale result

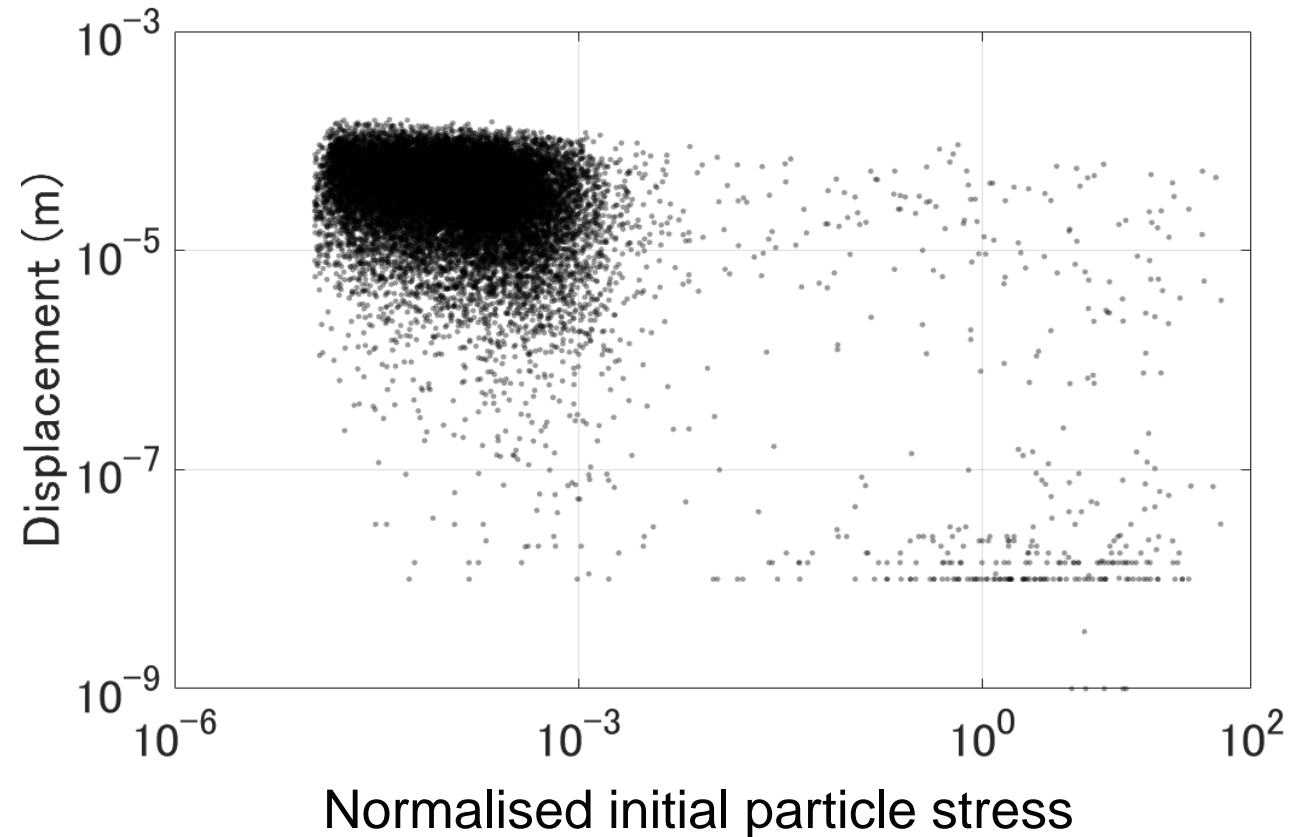
Results point towards different behaviour / fabric types:

Gap 25 Loose ($\alpha = 0.11$)

Coarse matrix
transfers stress



Fines do not participate in stress
transfer: Eroded *en masse*

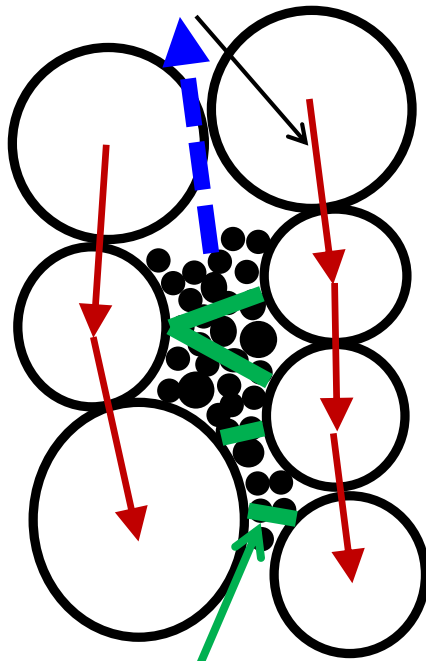


Coupled analysis: typical micro-scale result

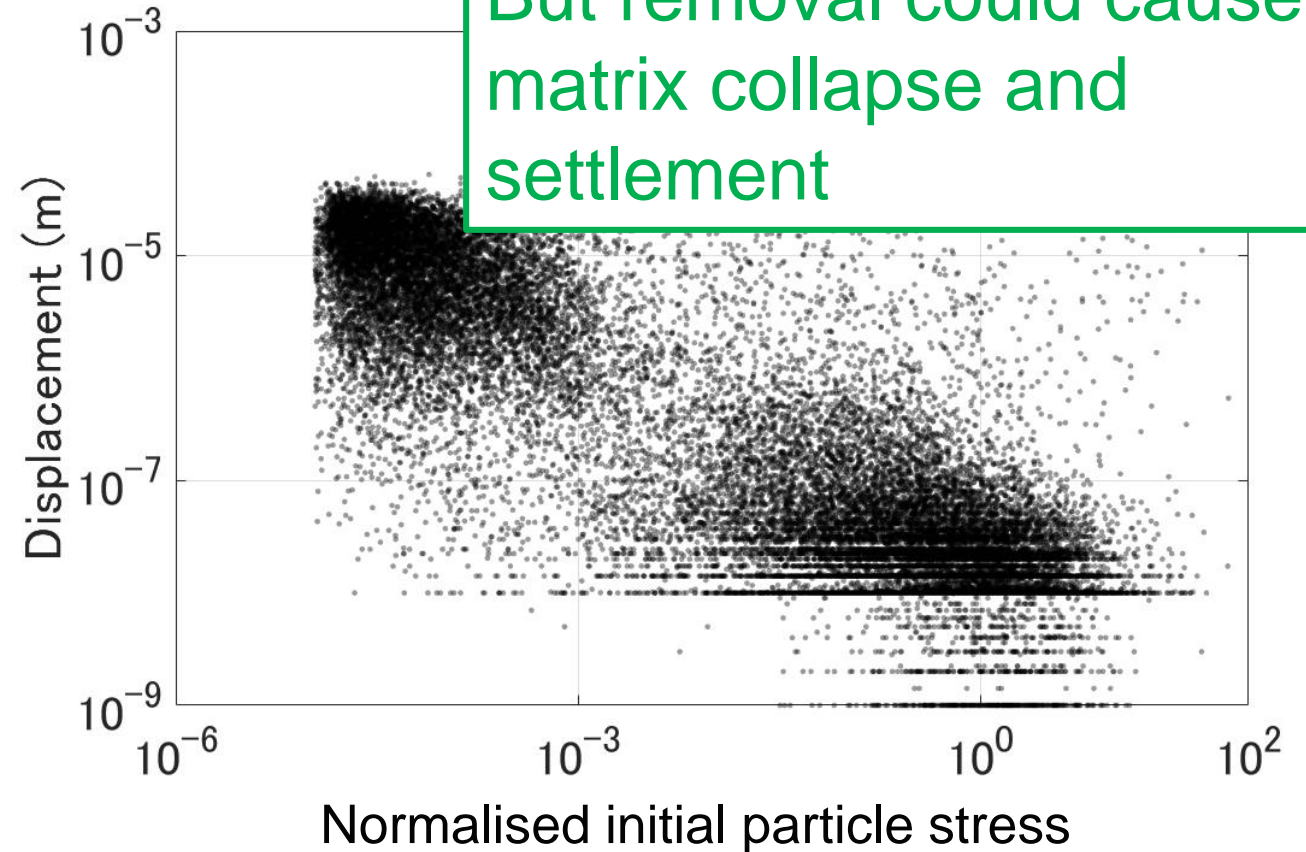
Results point towards different behaviour / fabric types:

Gap 35 Loose ($\alpha = 0.34$)

Coarse particles
dominate stress-transfer



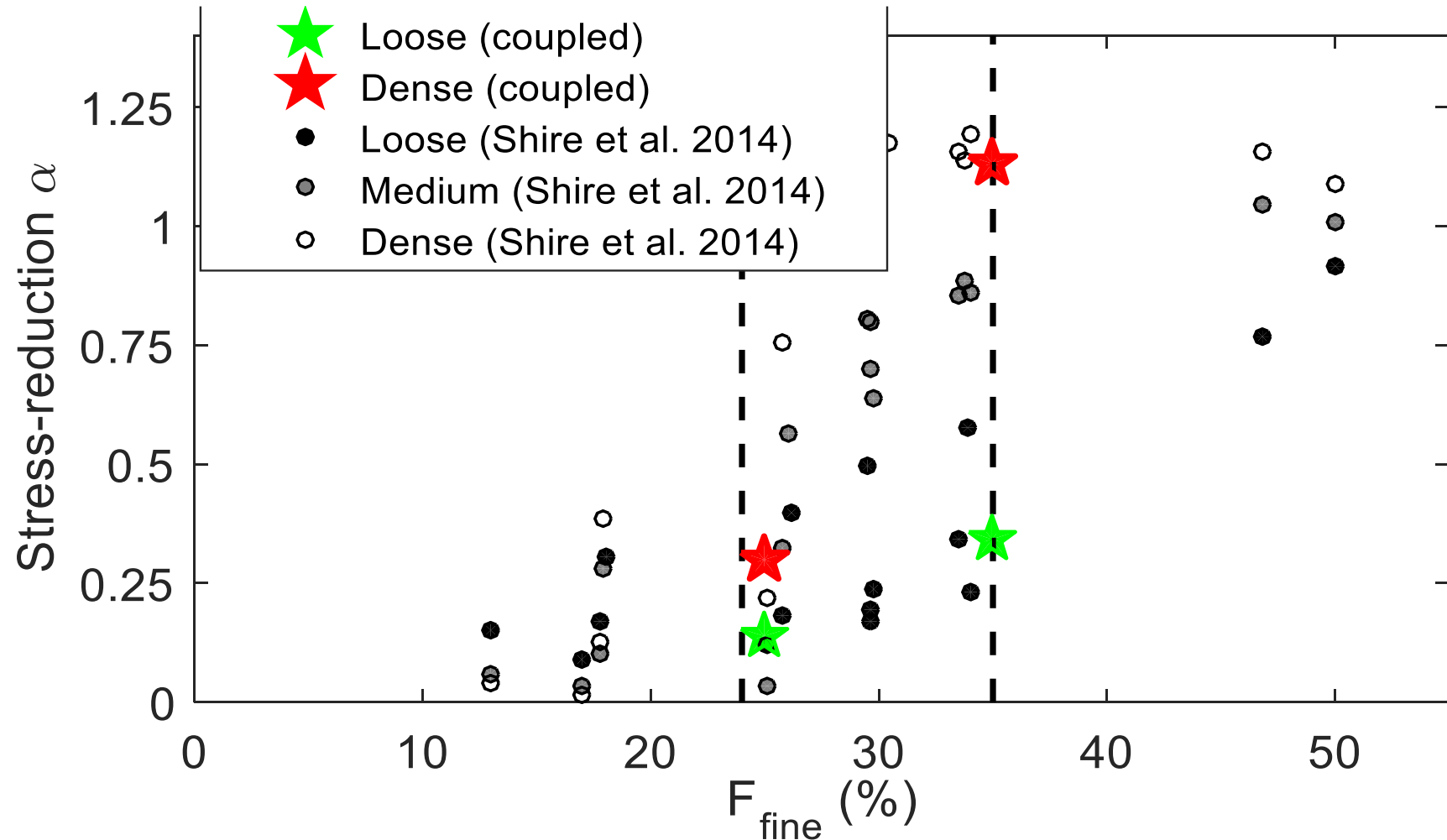
Understressed fines provide
lateral support to coarse matrix



Fines harder to remove:
But removal could cause
matrix collapse and
settlement

Coupled analysis: typical micro-scale result

Coupled study examined only small range of possible states (due to computational expense of simulations)



Summary

- DEM can be used to study internal instability at micro-scale
- Stress controls initiation of erosion and is linked to PSD and relative density
- Coupled DEM-CFD can be used to study fluid-particle interactions



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Acknowledgements



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Shire, T., O'Sullivan, C., Hanley, K. J., & Fannin, R. J. (2014). Fabric and effective stress distribution in internally unstable soils. *Journal of Geotechnical and Geoenvironmental Engineering*, 140(12), 04014072.